

7 Patricia Lane . Spring Valley, New York 10977 . (845) 570-0401

Congregation Kenneses Israel (50.05-1-5) New Hempstead, New York, 10977

DRAINAGE REPORT

Hydrologic assessment of the proposed development on offsite discharges

Date: 12/5/2024

WLE 23006

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Hydrologic Narrative, Methodology & Conclusion:

Hydrologic Narrative:

Congregation Knesses Israel is an existing congregation located at 698 Union Road in New Hempstead. The congregation seeks to subdivide the property and expand the parking lot. The existing residential structure on the site will be demolished and a new residential structure will be constructed on the new lot.

To achieve peak flow attenuation, an underground detention system is proposed under the expanded parking lot. All runoff from the Existing synagogue and parking areas will be routed through a pretreatment chamber into this system

A HydroCAD model of the existing and developed conditions was prepared to show the peak flow attenuation during the 1, 2, 5, 10, 25 and 100 year storm events.

Area Hydrology:

Under existing conditions, the 2.63-acre site generally drains towards one study point in the southwest corner of the site (Study Point "A") as indicated on the attached existing condition drainage area map. Under developed conditions, majority of site drains into existing curb inlet located southwest corner of the site and the balance drains to Study Point "A".

For the developed conditions, the site was configured into three distinct drainage areas, altering the flow patterns to distribute runoff more effectively. Drainage Area 1, 30,995 S.F., located on the western portion of the site, runs off towards the existing curb inlet at the southwest corner, Study Point A. Drainage Area 2, 45,205 S.F. encompasses the central part of the site, including the parking lot and driveways, and discharges into the newly proposed stormwater detention system. In the case of an extreme storm, twin 6" Ø pipes exit the system and drain to the catch basin at Study Point A. Drainage Area 3, 14,805 S.F. on the eastern portion, diverts runoff from the easternmost point around the site along the northern side to the furthest point northwest, where it discharges to the roadway and then later to Study Point A. This design ensures a reduction of peak runoff from existing conditions and improves overall site hydrology.

Methodology:

The developed site generally maintains the existing drainage pattern. Study Point "A", as indicated on the attached drainage maps, located at the Southwest corner of the property, was utilized to evaluate site runoff under pre and post development conditions.

All drainage area delineations and any changes from existing to proposed conditions are indicated on Existing and Proposed Drainage Maps provided in the Appendix.

Flows were established for existing and developed conditions utilizing the SCS Method.

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Time of concentration calculations were tabulated under existing and proposed conditions. Corresponding rainfall intensities for the design storms ranging from 1-year to 100-year were extracted from NOAA Point Precipitation Frequency Estimates. Supporting data is attached in the appendix.

Hydrologic analyses were revised for existing and proposed conditions utilizing SCS unit hydrographs.

The hydrograph calculations and summations were prepared using HydroCAD software. To establish these flows the 1, 2, 5, 10, 25 and 100 year, 24 hour storm precipitation values were derived from available NOAA data and incorporated in the HydroCAD models.

An underground detention chamber is proposed underneath the parking lot for peak flow attenuation, water quality and channel protection. The basin will be controlled by an outlet structure, which consists of two $6.0^{\circ}\Phi$ orifices.

The analysis indicates that the routed developed peak flows exiting the site at study point "A" are less than the existing peak flows for all storms ranging from the 2-year to the 100-year design frequencies.

Refer to attached HydroCAD reports for a comparison of flows produced under existing and developed conditions.

Conclusion:

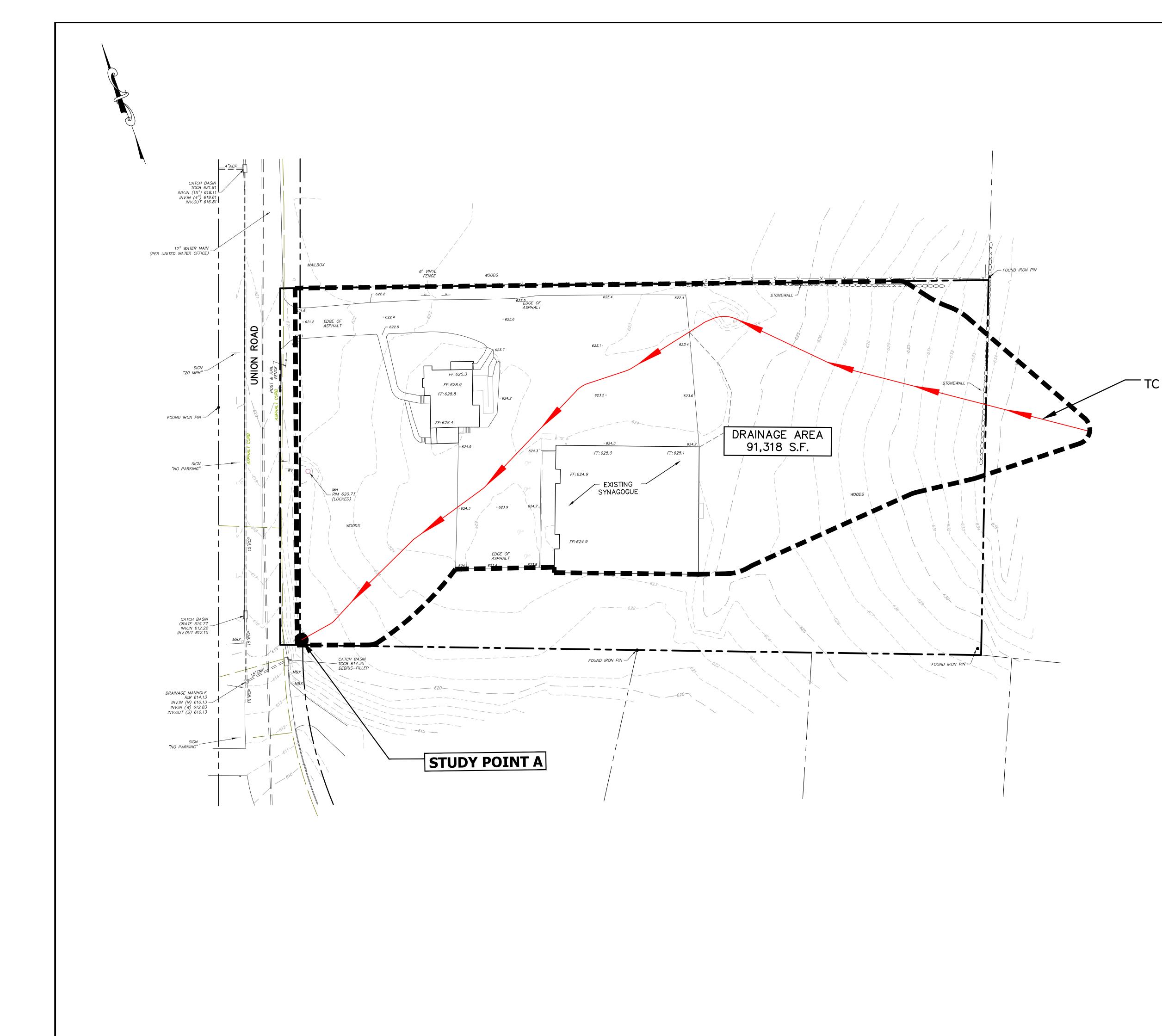
The following table shows the Peak Discharge rates for existing and developed conditions for the 1, 2, 5, 10, 25 and 100 year storm events. As shown, the peak discharges for all events for the developed conditions are less than those of the existing conditions.

	Peak Discharge (CFS)								
Storm	Existing	Developed							
Event	Conditions	Conditions							
1 Year	0.87	0.14							
2 Year	1.42	0.34							
5 Year	2.43	0.81							
10 Year	3.34	1.29							
25 Year	1.42	0.34							
100 Year	6.74	5.57							

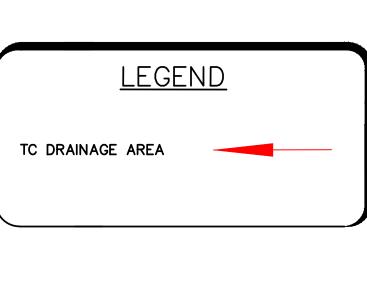
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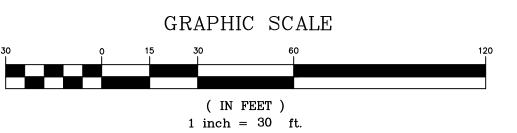
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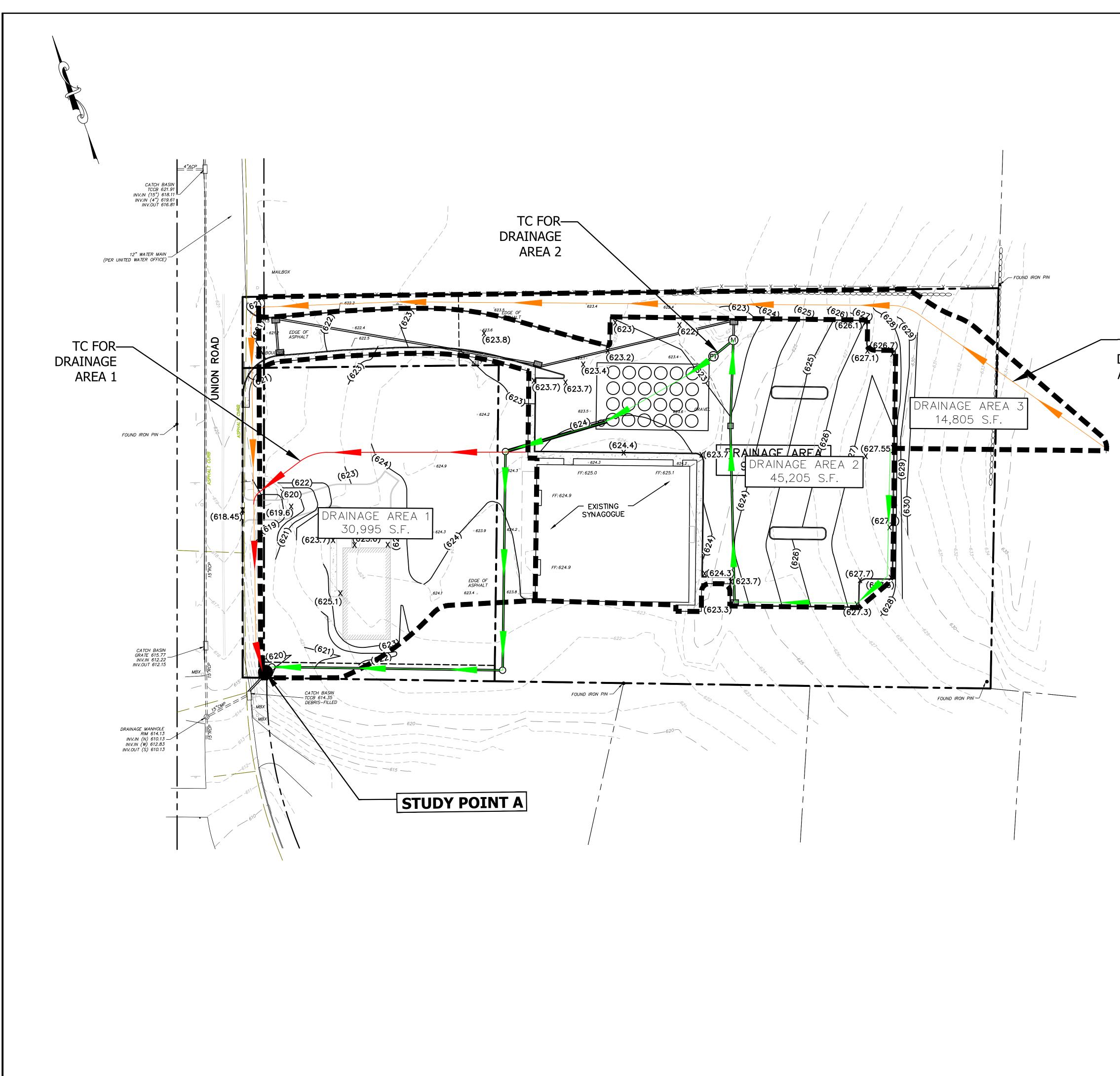
Appendix A



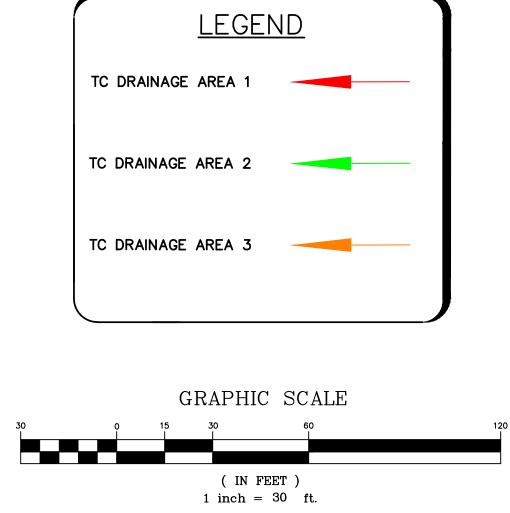
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K 0/: 30	ROCKLAND COUNTY, NEW YORK				
5		PRING VALLEY, NEW YORK 10977		REVISED BULK TABLE	05/09/24
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TC FOR DRAINAGE AREA 3

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Appendix B



NOAA Atlas 14, Volume 10, Version 3 Location name: Spring Valley, New York, USA* Latitude: 41.1406°, Longitude: -74.0552° Elevation: 625 ft** * source: ESRI Maps ** source: USGS



POINT PRECIPITATION FREQUENCY ESTIMATES

Sanja Perica, Sandra Pavlovic, Michael St. Laurent, Carl Trypaluk, Dale Unruh, Orlan Wilhite

NOAA, National Weather Service, Silver Spring, Maryland

PF_tabular | PF_graphical | Maps_&_aerials

PF tabular

PDS-b	PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches) ¹										
Duration				Average	recurrence	interval (y	ears)				
Duration	1	2	5	10	25	50	100	200	500	1000	
5-min	0.369 (0.287-0.465)	0.435 (0.338-0.549)	0.543 (0.421-0.688)	0.633 (0.487-0.805)	0.756 (0.563-1.00)	0.850 (0.621-1.15)	0.947 (0.668-1.32)	1.05 (0.708-1.50)	1.19 (0.771-1.76)	1.30 (0.823-1.96)	
10-min	0.522 (0.406-0.659)	0.616 (0.479-0.778)	0.769 (0.596-0.975)	0.896 (0.690-1.14)	1.07 (0.798-1.42)	1.20 (0.878-1.62)	1.34 (0.947-1.86)	1.49 (1.00-2.12)	1.68 (1.09-2.49)	1.84 (1.17-2.77)	
15-min	0.614 (0.478-0.775)	0.725 (0.563-0.916)	0.905 (0.701-1.15)	1.06 (0.812-1.34)	1.26 (0.939-1.67)	1.42 (1.03-1.91)	1.58 (1.11-2.19)	1.75 (1.18-2.50)	1.98 (1.28-2.92)	2.17 (1.37-3.26)	
30-min	0.848 (0.660-1.07)	0.998 (0.776-1.26)	1.24 (0.963-1.58)	1.45 (1.11-1.84)	1.73 (1.29-2.28)	1.94 (1.42-2.62)	2.16 (1.52-3.00)	2.39 (1.61-3.42)	2.71 (1.76-4.00)	2.96 (1.87-4.45)	
60-min	1.08 (0.841-1.36)	1.27 (0.988-1.61)	1.58 (1.23-2.00)	1.84 (1.42-2.34)	2.20 (1.64-2.90)	2.47 (1.80-3.32)	2.74 (1.94-3.81)	3.03 (2.05-4.33)	3.43 (2.22-5.06)	3.74 (2.37-5.63)	
2-hr	1.45 (1.14-1.82)	1.68 (1.32-2.11)	2.06 (1.61-2.60)	2.38 (1.85-3.01)	2.82 (2.11-3.70)	3.15 (2.31-4.21)	3.49 (2.48-4.82)	3.85 (2.61-5.46)	4.36 (2.84-6.39)	4.76 (3.02-7.11)	
3-hr	1.69 (1.33-2.11)	1.96 (1.54-2.45)	2.41 (1.89-3.02)	2.78 (2.16-3.50)	3.29 (2.48-4.30)	3.67 (2.71-4.90)	4.07 (2.91-5.62)	4.51 (3.06-6.37)	5.12 (3.34-7.47)	5.61 (3.57-8.35)	
6-hr	2.11 (1.67-2.61)	2.49 (1.97-3.09)	3.12 (2.46-3.88)	3.64 (2.85-4.56)	4.35 (3.30-5.68)	4.89 (3.63-6.51)	5.46 (3.92-7.51)	6.09 (4.14-8.55)	6.99 (4.57-10.1)	7.72 (4.93-11.4)	
12-hr	2.52 (2.01-3.10)	3.07 (2.45-3.78)	3.97 (3.15-4.91)	4.71 (3.72-5.86)	5.74 (4.38-7.45)	6.51 (4.86-8.62)	7.32 (5.31-10.0)	8.25 (5.64-11.5)	9.59 (6.29-13.8)	10.7 (6.84-15.7)	
24-hr	2.94 (2.36-3.59)	3.64 (2.92-4.46)	4.78 (3.83-5.88)	5.73 (4.56-7.08)	7.04 (5.41-9.09)	8.02 (6.03-10.6)	9.06 (6.61-12.4)	10.3 (7.04-14.2)	<mark>12.0</mark> (7.91-17.2)	13.5 (8.66-19.7)	
2-day	3.39 (2.75-4.13)	4.17 (3.38-5.08)	5.44 (4.39-6.65)	6.50 (5.21-7.98)	7.96 (6.16-10.2)	9.04 (6.84-11.8)	10.2 (7.50-13.9)	11.6 (7.96-15.9)	13.6 (8.97-19.3)	15.3 (9.84-22.1)	
3-day	3.72 (3.03-4.51)	4.53 (3.68-5.50)	5.86 (4.75-7.13)	6.97 (5.60-8.52)	8.48 (6.59-10.8)	9.61 (7.31-12.6)	10.8 (7.99-14.7)	12.3 (8.46-16.8)	14.4 (9.54-20.4)	16.2 (10.5-23.4)	
4-day	4.00 (3.26-4.83)	4.84 (3.95-5.85)	6.22 (5.05-7.54)	7.36 (5.94-8.97)	8.93 (6.96-11.4)	10.1 (7.70-13.2)	11.4 (8.40-15.4)	12.8 (8.88-17.6)	15.1 (10.0-21.3)	17.0 (11.0-24.5)	
7-day	4.74	5.66	7.15	8.40	10.1	11.4	12.7	14.4	16.8	18.8	

https://hdsc.nws.noaa.gov/pfds/pfds_printpage.html?lat=41.1406&lon=-74.0552&data=depth&units=english&series=pds

Precipitation Frequency Data Server

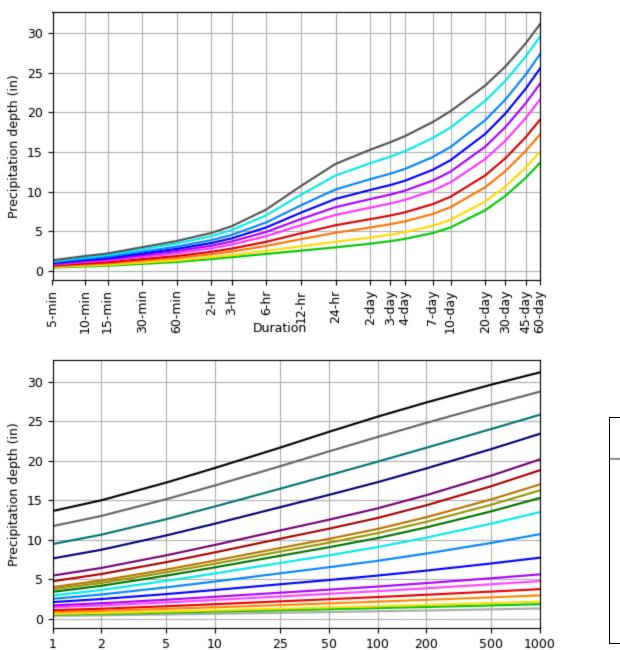
	(3.89-5.70)	(4.64-6.80)	(5.84-8.63)	(6.81-10.2)	(7.92-12.8)	(8.71-14.7)	(9.46-17.1)	(9.96-19.5)	(11.1-23.5)	(12.2-26.9)
10-day	5.45 (4.49-6.52)	6.42 (5.29-7.70)	8.02 (6.57-9.64)	9.34 (7.61-11.3)	11.2 (8.76-14.1)	12.5 (9.60-16.1)	14.0 (10.4-18.6)	15.6 (10.9-21.2)	18.1 (12.1-25.3)	20.2 (13.1-28.7)
20-day	7.62 (6.33-9.06)	8.73 (7.24-10.4)	10.5 (8.70-12.6)	12.0 (9.87-14.4)	14.1 (11.1-17.5)	15.7 (12.0-19.9)	17.3 (12.8-22.6)	19.0 (13.3-25.6)	21.5 (14.4-29.8)	23.4 (15.2-33.1)
30-day	9.44 (7.88-11.2)	10.6 (8.86-12.6)	12.6 (10.4-15.0)	14.2 (11.7-17.0)	16.4 (13.0-20.3)	18.1 (14.0-22.8)	19.9 (14.7-25.8)	21.6 (15.2-28.9)	24.0 (16.1-33.2)	25.8 (16.8-36.4)
45-day	11.7 (9.81-13.8)	13.0 (10.9-15.3)	15.1 (12.6-17.9)	16.9 (14.0-20.1)	19.3 (15.3-23.7)	21.2 (16.3-26.4)	23.0 (17.0-29.5)	24.8 (17.5-33.0)	27.1 (18.2-37.2)	28.7 (18.8-40.4)
60-day	13.6 (11.4-16.0)	15.0 (12.6-17.6)	17.2 (14.4-20.3)	19.1 (15.8-22.6)	21.6 (17.2-26.4)	23.6 (18.3-29.4)	25.6 (18.9-32.6)	27.4 (19.4-36.3)	29.6 (20.0-40.6)	31.2 (20.4-43.7)

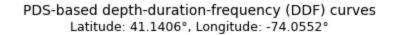
¹ Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).

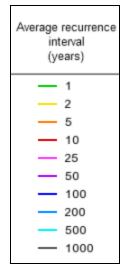
Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values. Please refer to NOAA Atlas 14 document for more information.

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PF graphical







Duration								
— 5-min	2-day							
	- 3-day							
- 15-min	4-day							
	- 7-day							
- 60-min	— 10-day							
2-hr	— 20-day							
— 3-hr	— 30-day							
6-hr	— 45-day							
12-hr	- 60-day							
24-hr								

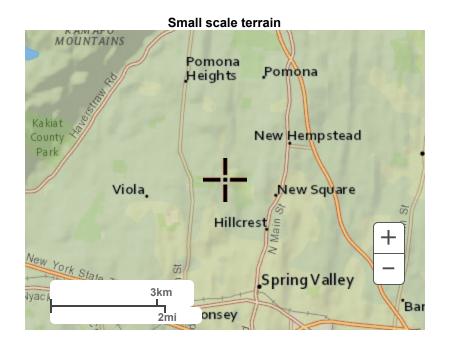
Average recurrence interval (years)

NOAA Atlas 14, Volume 10, Version 3

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Maps & aerials



Large scale terrain

Precipitation Frequency Data Server

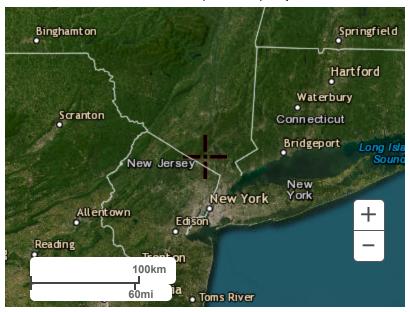


Large scale map



Large scale aerial

Precipitation Frequency Data Server



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US Department of Commerce National Oceanic and Atmospheric Administration National Weather Service National Water Center 1325 East West Highway Silver Spring, MD 20910 Questions?: HDSC.Questions@noaa.gov

Disclaimer



USDA Natural Resources Conservation Service Web Soil Survey National Cooperative Soil Survey

MAP L	EGEND	MAP INFORMATION			
Area of Interest (AOI)	🚍 Spoil Area	The soil surveys that comprise your AOI were mapped at			
Area of Interest (AOI)	Stony Spot	1:24,000.			
Soils	Very Stony Spot	Warning: Soil Map may not be valid at this scale.			
Soil Map Unit Polygons	Wet Spot	Enlargement of maps beyond the scale of mapping can cause			
Map Unit Lines	of ther of the the term of term o	misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of			
Soil Map Unit Points	Special Line Features	contrasting soils that could have been shown at a more detailed			
Special Point Features	Water Features	scale.			
Blowout	Streams and Canals	Please rely on the bar scale on each map sheet for map			
Borrow Pit	Transportation	measurements.			
💥 Clay Spot	+++ Rails	Source of Map: Natural Resources Conservation Service			
Closed Depression	nterstate Highways	Web Soil Survey URL: Coordinate System: Web Mercator (EPSG:3857)			
💥 Gravel Pit	JS Routes	Maps from the Web Soil Survey are based on the Web Mercato			
Gravelly Spot	Major Roads	projection, which preserves direction and shape but distorts			
🚯 Landfill	Local Roads	distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more			
Lava Flow	Background	accurate calculations of distance or area are required.			
Marsh or swamp	Aerial Photography	This product is generated from the USDA-NRCS certified data a of the version date(s) listed below.			
Mine or Quarry		Soil Survey Area: Rockland County, New York			
Miscellaneous Water		Survey Area Data: Version 21, Sep 6, 2023			
Perennial Water		Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.			
Rock Outcrop					
Saline Spot		Date(s) aerial images were photographed: May 31, 2022—Oc 27, 2022			
Sandy Spot		The orthophoto or other base map on which the soil lines were			
Severely Eroded Spot		compiled and digitized probably differs from the background			
Sinkhole		imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.			
Slide or Slip					
Sodic Spot					



Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
CrB	Cheshire gravelly fine sandy loam, 2 to 8 percent slopes	4.6	100.0%
Totals for Area of Interest		4.6	100.0%



Rockland County, New York

CrB—Cheshire gravelly fine sandy loam, 2 to 8 percent slopes

Map Unit Setting

National map unit symbol: 9v46 Elevation: 50 to 670 feet Mean annual precipitation: 47 to 50 inches Mean annual air temperature: 48 to 52 degrees F Frost-free period: 135 to 215 days Farmland classification: All areas are prime farmland

Map Unit Composition

Cheshire and similar soils: 80 percent Minor components: 20 percent Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Cheshire

Setting

Landform: Hills, till plains Landform position (two-dimensional): Summit Landform position (three-dimensional): Crest Down-slope shape: Convex Across-slope shape: Convex Parent material: Loamy till derived mostly from reddish sandstone, shale, and conglomerate

Typical profile

H1 - 0 to 10 inches: gravelly fine sandy loam *H2 - 10 to 22 inches:* gravelly fine sandy loam *H3 - 22 to 60 inches:* gravelly sandy loam

Properties and qualities

Slope: 3 to 8 percent Depth to restrictive feature: More than 80 inches Drainage class: Well drained Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high (0.57 to 5.95 in/hr) Depth to water table: More than 80 inches Frequency of flooding: None Frequency of ponding: None Available water supply, 0 to 60 inches: Moderate (about 8.1 inches)

Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 2e Hydrologic Soil Group: B Ecological site: F145XY013CT - Well Drained Till Uplands

USDA

Hydric soil rating: No

Minor Components

Watchaug

Percent of map unit: 5 percent Hydric soil rating: No

Cheshire, very stony Percent of map unit: 5 percent Hydric soil rating: No

Wethersfield

Percent of map unit: 5 percent *Hydric soil rating:* No

Yalesville

Percent of map unit: 5 percent *Hydric soil rating:* No

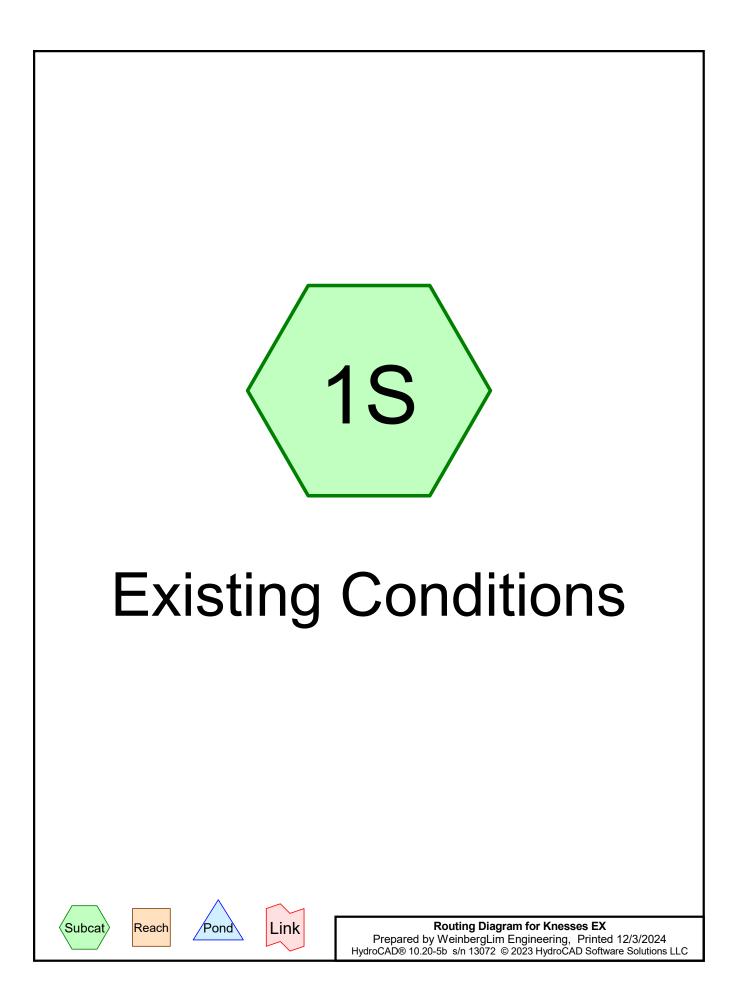
Data Source Information

Soil Survey Area: Rockland County, New York Survey Area Data: Version 21, Sep 6, 2023

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Appendix C



Project Notes

Rainfall events imported from "Knesses DEV.hcp"

Event#	Event Name	Storm Type	Curve	Mode	Duration (hours)	B/B	Depth (inches)	AMC
1	Spring Valley 10yr	Type III 24-hr		Default	24.00	1	5.73	2
2	Spring Valley 100yr	Type III 24-hr		Default	24.00	1	9.06	2
3	Spring Valley 1yr	Type III 24-hr		Default	24.00	1	2.94	2
4	Spring Valley 2yr	Type III 24-hr		Default	24.00	1	3.64	2
5	Spring Valley 5yr	Type III 24-hr		Default	24.00	1	4.78	2
6	Spring Valley 25yr	Type III 24-hr		Default	24.00	1	3.64	2

Rainfall Events Listing

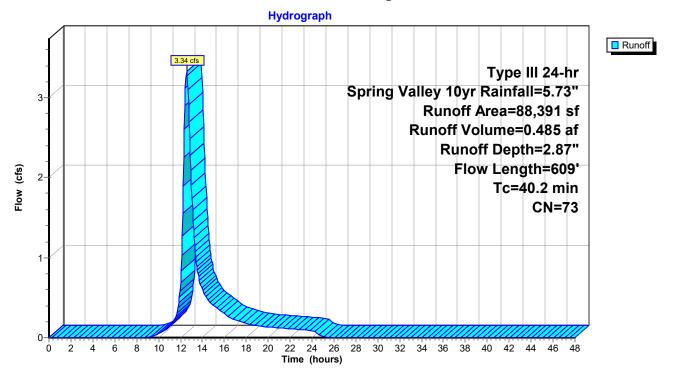
Soil Listing (selected nodes)

Area	Soil	Subcatchment
(acres)	Group	Numbers
0.000	HSG A	
2.029	HSG B	1S
0.000	HSG C	
0.000	HSG D	
0.000	Other	
2.029		TOTAL AREA

Runoff = 3.34 cfs @ 12.56 hrs, Volume= 0.485 af, Depth= 2.87"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type III 24-hr Spring Valley 10yr Rainfall=5.73"

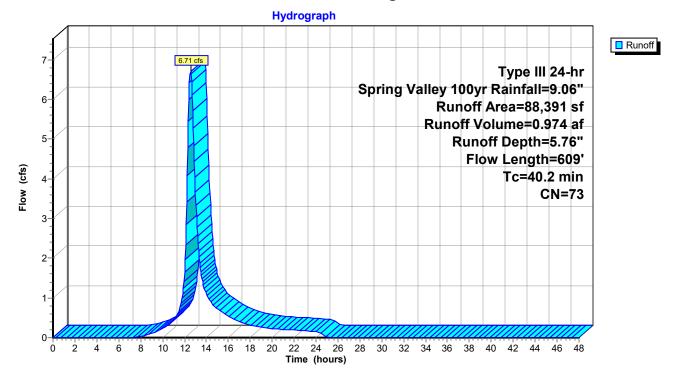
A	rea (sf)	CN E	Description					
	35,091	60 V	Voods, Fai	r, HSG B				
	29,574	98 F	Paved park	ing, HSG B				
	23,726	61 >	75% Gras	s cover, Go	ood, HSG B			
	88,391	73 V	Veighted A	verage				
	58,817	6	66.54% Pervious Area					
	29,574	3	3.46% Imp	ervious Ar	ea			
Тс	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
28.2	270	0.0630	0.16		Sheet Flow, woods in back			
					Woods: Light underbrush n= 0.400 P2= 3.64"			
2.1	194	0.0155	1.51		Sheet Flow, parking lot			
					Smooth surfaces n= 0.011 P2= 3.64"			
9.9	145	0.0345	0.24		Sheet Flow, grass in front			
					Grass: Short n= 0.150 P2= 3.64"			
40.2	609	Total						



Runoff = 6.71 cfs @ 12.55 hrs, Volume= 0.974 af, Depth= 5.76"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type III 24-hr Spring Valley 100yr Rainfall=9.06"

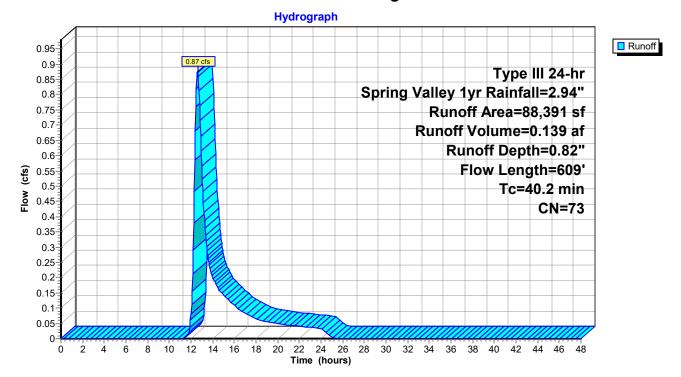
A	rea (sf)	CN E	Description					
	35,091	60 V	Voods, Fai	r, HSG B				
	29,574	98 F	Paved park	ing, HSG B				
	23,726	61 >	•75% Gras	s cover, Go	ood, HSG B			
	88,391	73 V	Veighted A	verage				
	58,817	6	66.54% Pervious Area					
	29,574	3	3.46% Imp	pervious Ar	ea			
Тс	Length	Slope		Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
28.2	270	0.0630	0.16		Sheet Flow, woods in back			
					Woods: Light underbrush n= 0.400 P2= 3.64"			
2.1	194	0.0155	1.51		Sheet Flow, parking lot			
					Smooth surfaces n= 0.011 P2= 3.64"			
9.9	145	0.0345	0.24		Sheet Flow, grass in front			
					Grass: Short n= 0.150 P2= 3.64"			
40.2	609	Total						



Runoff = 0.87 cfs @ 12.62 hrs, Volume= 0.139 af, Depth= 0.82"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type III 24-hr Spring Valley 1yr Rainfall=2.94"

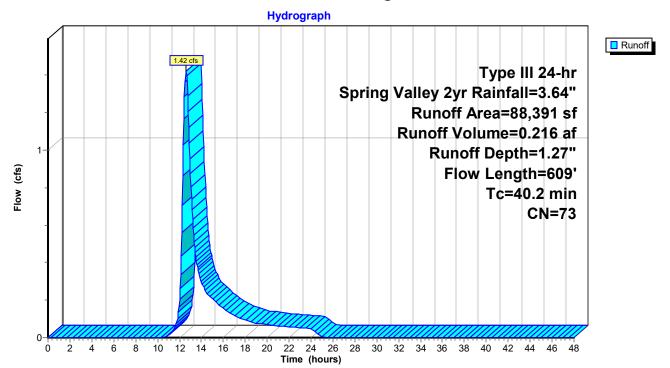
_	A	rea (sf)	CN E	Description			
	35,091 60 Woods, Fair, HSG B						
29,574 98 Paved parking, HSG B							
_	23,726 61 >75% Grass cover, Good, HSG B						
		88,391	73 V	Veighted A	verage		
		58,817	6	6.54% Per	vious Area		
		29,574	3	3.46% Imp	pervious Ar	ea	
	Тс	Length	Slope	Velocity	Capacity	Description	
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
	28.2	270	0.0630	0.16		Sheet Flow, woods in back	
						Woods: Light underbrush n= 0.400 P2= 3.64"	
	2.1	194	0.0155	1.51		Sheet Flow, parking lot	
						Smooth surfaces n= 0.011 P2= 3.64"	
	9.9	145	0.0345	0.24		Sheet Flow, grass in front	
_						Grass: Short n= 0.150 P2= 3.64"	
	40.2	609	Total				



Runoff = 1.42 cfs @ 12.59 hrs, Volume= 0.216 af, Depth= 1.27"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type III 24-hr Spring Valley 2yr Rainfall=3.64"

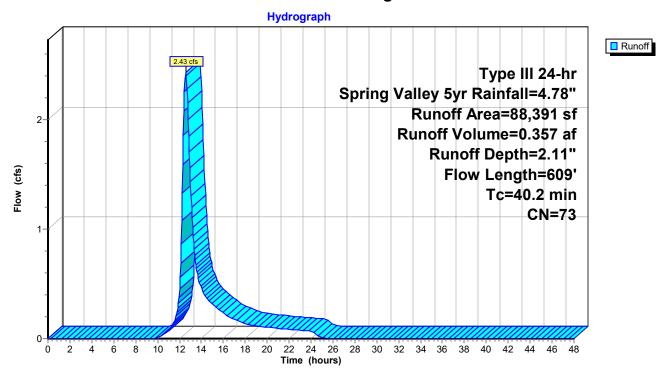
	Area (sf)	CN [Description			
	35,091 60 Woods, Fair, HSG B					
	29,574 98 Paved parking, HSG B					
	23,726	61 >	>75% Gras	s cover, Go	ood, HSG B	
	88,391 73 Weighted Average					
	58,817	6	6.54% Per	vious Area		
	29,574	3	33.46% Imp	pervious Ar	ea	
То	5	Slope		Capacity	Description	
(min) (feet)	(ft/ft)	(ft/sec)	(cfs)		
28.2	2 270	0.0630	0.16		Sheet Flow, woods in back	
					Woods: Light underbrush n= 0.400 P2= 3.64"	
2.1	l 194	0.0155	1.51		Sheet Flow, parking lot	
					Smooth surfaces n= 0.011 P2= 3.64"	
9.9	9 145	0.0345	0.24		Sheet Flow, grass in front	
					Grass: Short n= 0.150 P2= 3.64"	
40.2	2 609	Total				



Runoff = 2.43 cfs @ 12.57 hrs, Volume= 0.357 af, Depth= 2.11"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type III 24-hr Spring Valley 5yr Rainfall=4.78"

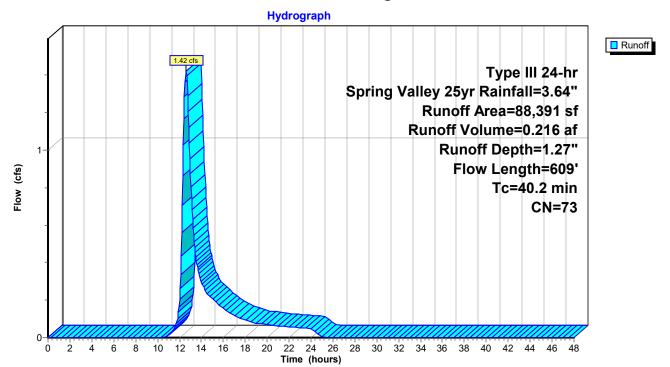
	Area (sf)	CN [Description			
	35,091 60 Woods, Fair, HSG B					
	29,574 98 Paved parking, HSG B					
	23,726	61 >	>75% Gras	s cover, Go	ood, HSG B	
	88,391 73 Weighted Average					
	58,817	6	6.54% Per	vious Area		
	29,574	3	33.46% Imp	pervious Ar	ea	
То	5	Slope		Capacity	Description	
(min) (feet)	(ft/ft)	(ft/sec)	(cfs)		
28.2	2 270	0.0630	0.16		Sheet Flow, woods in back	
					Woods: Light underbrush n= 0.400 P2= 3.64"	
2.1	l 194	0.0155	1.51		Sheet Flow, parking lot	
					Smooth surfaces n= 0.011 P2= 3.64"	
9.9	9 145	0.0345	0.24		Sheet Flow, grass in front	
					Grass: Short n= 0.150 P2= 3.64"	
40.2	2 609	Total				



Runoff = 1.42 cfs @ 12.59 hrs, Volume= 0.216 af, Depth= 1.27"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type III 24-hr Spring Valley 25yr Rainfall=3.64"

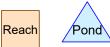
A	rea (sf)	CN E	Description			
	35,091 60 Woods, Fair, HSG B					
	29,574	98 F	Paved park	ing, HSG B		
	23,726	61 >	75% Gras	s cover, Go	ood, HSG B	
	88,391	73 V	Veighted A	verage		
	58,817	6	6.54% Per	vious Area		
	29,574	3	3.46% Imp	pervious Are	ea	
Тс	Length	Slope	Velocity	Capacity	Description	
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
28.2	270	0.0630	0.16		Sheet Flow, woods in back	
					Woods: Light underbrush n= 0.400 P2= 3.64"	
2.1	194	0.0155	1.51		Sheet Flow, parking lot	
					Smooth surfaces n= 0.011 P2= 3.64"	
9.9	145	0.0345	0.24		Sheet Flow, grass in front	
					Grass: Short n= 0.150 P2= 3.64"	
40.2	609	Total				





Proposed Conditions





Link

Routing Diagram for Knesses TEST1 DEV-AW Prepared by WeinbergLim Engineering, Printed 12/3/2024 HydroCAD® 10.20-5b s/n 13072 © 2023 HydroCAD Software Solutions LLC

Project Notes

Rainfall events imported from "Knesses EX.hcp" Rainfall events imported from "Knesses EX.hcp"

Event#	Event Name	Storm Type	Curve	Mode	Duration (hours)	B/B	Depth (inches)	AMC
1	Spring Valley 10yr	Type III 24-hr		Default	24.00	1	5.73	2
2	Spring Valley 100yr	Type III 24-hr		Default	24.00	1	9.06	2
3	Spring Valley 1yr	Type III 24-hr		Default	24.00	1	2.94	2
4	Spring Valley 2yr	Type III 24-hr		Default	24.00	1	3.64	2
5	Spring Valley 5yr	Type III 24-hr		Default	24.00	1	4.78	2
6	Spring Valley 25yr	Type III 24-hr		Default	24.00	1	3.64	2

Rainfall Events Listing

Soil Listing (selected nodes)

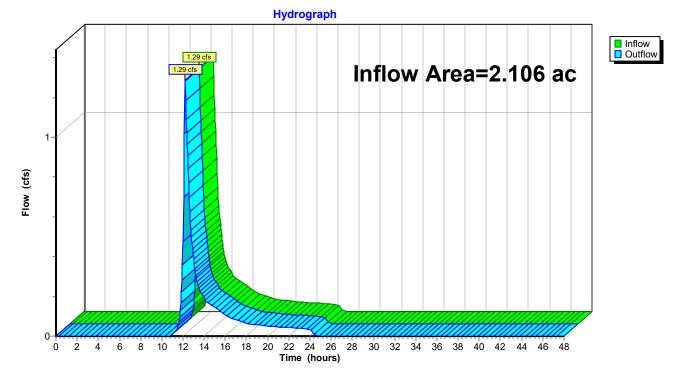
Area	Soil	Subcatchment
(acres)	Group	Numbers
0.000	HSG A	
0.000	HSG B	
0.000	HSG C	
0.000	HSG D	
0.000	Other	
0.000		TOTAL AREA

Summary for Reach 3R: Proposed Conditions

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area	a =	2.106 ac, 50.73% Impervious, Inflow Depth = 0.89" for Spring Valley 10yr event
Inflow	=	1.29 cfs @ 12.19 hrs, Volume= 0.156 af
Outflow	=	1.29 cfs @ 12.19 hrs, Volume= 0.156 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs



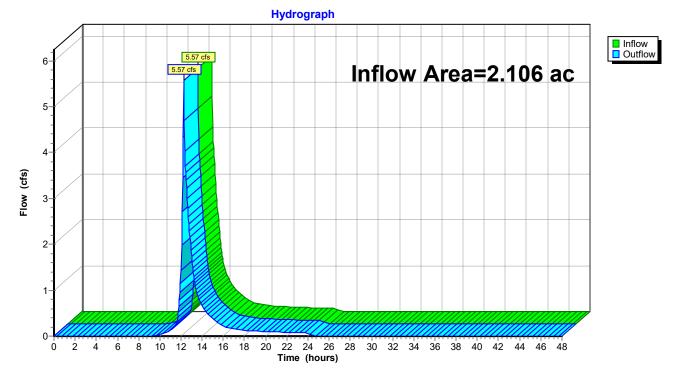
Reach 3R: Proposed Conditions

Summary for Reach 3R: Proposed Conditions

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area :	=	2.106 ac, 50.73% Impervious, Inflow Depth = 2.96" for Spring Valley 100yr event
Inflow =	=	5.57 cfs @ 12.25 hrs, Volume= 0.519 af
Outflow =	=	5.57 cfs @ 12.25 hrs, Volume= 0.519 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs

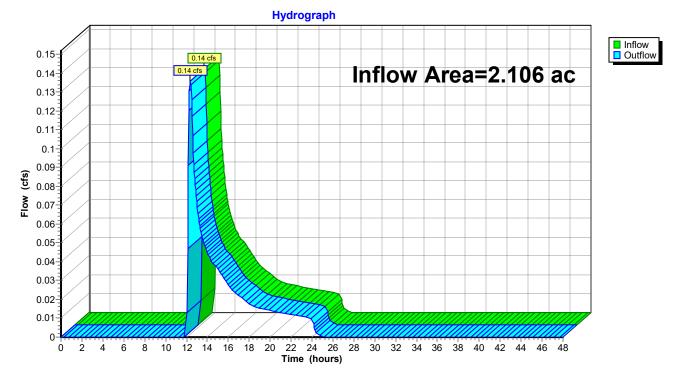


Reach 3R: Proposed Conditions

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area =	2.106 ac, 50.73% Impervious, Int	flow Depth = 0.16" for Spring Valley 1yr event	
Inflow =	0.14 cfs @ 12.37 hrs, Volume=	0.029 af	
Outflow =	0.14 cfs @ 12.37 hrs, Volume=	0.029 af, Atten= 0%, Lag= 0.0 min	

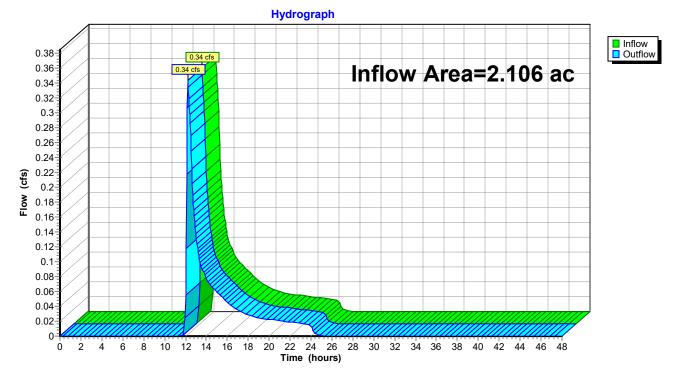
Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs



[40] Hint: Not Described (Outflow=Inflow)

Inflow Area	a =	2.106 ac, 50.73% Impervious, Inflow Depth = 0.31" for Spring Valley 2yr event
Inflow	=	0.34 cfs @ 12.22 hrs, Volume= 0.054 af
Outflow	=	0.34 cfs @ 12.22 hrs, Volume= 0.054 af, Atten= 0%, Lag= 0.0 min

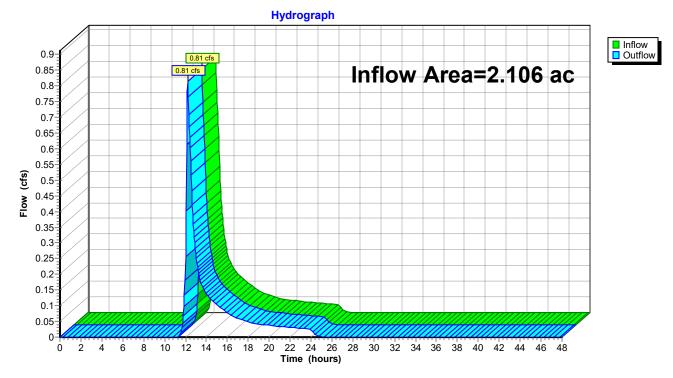
Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs



[40] Hint: Not Described (Outflow=Inflow)

Inflow Are	a =	2.106 ac, 50.73% Impervious, Inflow Depth = 0.60" for Spring Valley 5yr event
Inflow	=	0.81 cfs @ 12.19 hrs, Volume= 0.105 af
Outflow	=	0.81 cfs @ 12.19 hrs, Volume= 0.105 af, Atten= 0%, Lag= 0.0 min

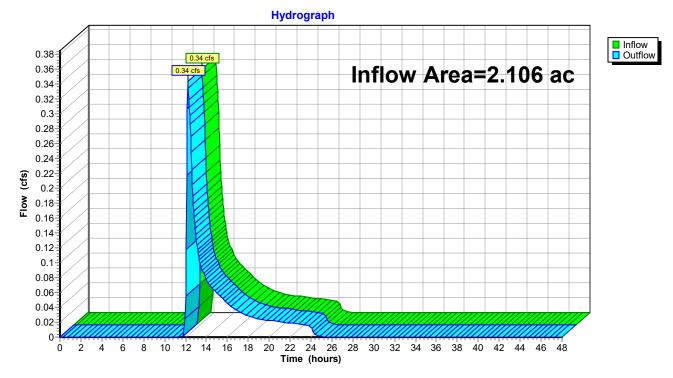
Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs



[40] Hint: Not Described (Outflow=Inflow)

Inflow Area	a =	2.106 ac, 50.73% Impervious, Inflow Depth = 0.31" for Spring Valley 25yr event
Inflow	=	0.34 cfs @ 12.22 hrs, Volume= 0.054 af
Outflow	=	0.34 cfs @ 12.22 hrs, Volume= 0.054 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs



WeinbergLim Engineering

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Appendix D

WQv & RRv Calculations

ProjectCongregation Knesses IsraelJob #23005Date12/3/2024

WQv	P*Rv*A/12		RRvmin	P*Rv*Aic*S/12	
WQv	0.0141	Acre feet	RRvmin	0.0015	Acre feet
Р	1.5	in	Ρ	1.5	in
1	48.18%		1	100.00%	
Rv	0.054335792		Rv	0.059	
А	2.08	Acres	Aic	0.50	
			S (B)	0.4	
Total Drainage Area	90,497	SF			
			Increased Impervious Area	21,875	SF
Lot B Area	85,485	SF		0.50	Acres
Impervious Area	43,597	SF			



State of New Jersey

DEPARTMENT OF ENVIRONMENTAL PROTECTION

Bureau of NJPDES Stormwater Permitting & Water Quality Management Division of Watershed Protection and Restoration 401-02B Post Office Box 420 Trenton, New Jersey 08625-0420 609-633-7021 Fax: 609-777-0432 SHAWN M. LATOURETTE Acting Commissioner

April 28, 2021

Daniel J. Figola, P.E. Director of Sustainability Development Advanced Drainage Systems, Inc. 1030 Deer Hollow Drive Mt. Airy, MD 21771

Re: MTD Lab Certification BarracudaTM MAX Hydrodynamic Separator Stormwater Treatment Device On-line Installation

TSS Removal Rate 50%

Dear Mr. Figola:

The Stormwater Management rules under N.J.A.C. 7:8-5.5(b) and 5.7(c) allow the use of manufactured treatment devices (MTDs) for compliance with the design and performance standards at N.J.A.C. 7:8-5 if the pollutant removal rates have been verified by the New Jersey Corporation for Advanced Technology (NJCAT) and have been certified by the New Jersey Department of Environmental Protection (NJDEP). Advanced Drainage Systems, Inc. (ADS) has requested an MTD Laboratory Certification for the BarracudaTM MAX Hydrodynamic Separator stormwater treatment system (BarracudaTM MAX).

The project falls under the "Procedure for Obtaining Verification of a Stormwater Manufactured Treatment Device from New Jersey Corporation for Advance Technology" dated January 25, 2013. The applicable protocol is the "New Jersey Laboratory Testing Protocol to Assess Total Suspended Solids Removal by a Hydrodynamic Sedimentation Manufactured Treatment Device" dated January 25, 2013.

NJCAT verification documents submitted to the NJDEP indicate that the requirements of the aforementioned protocol have been met or exceeded. The NJCAT letter also included a recommended certification TSS removal rate and the required maintenance plan. The NJCAT Verification Report with the Verification Appendix (dated April 2021) for this device is published online at http://www.njcat.org/verification-process/technology-verification-process/technology-verification-database.html.

PHILIP D. MURPHY Governor

SHEILA Y. OLIVER Lt. Governor The NJDEP certifies the use of the Barracuda[™] MAX stormwater treatment system at a TSS removal rate of 50% when designed, operated, and maintained in accordance with the information provided in the Verification Appendix and the following conditions:

- 1. The maximum treatment flow rate (MTFR) for the manufactured treatment device (MTD) is calculated using the New Jersey Water Quality Design Storm (1.25 inches in 2 hrs) in N.J.A.C. 7:8-5.5.
- 2. The BarracudaTM MAX shall be installed using the same configuration reviewed by NJCAT and shall be sized in accordance with the criteria specified in item 6 below.
- 3. This BarracudaTM MAX cannot be used in series with another MTD or a media filter (such as a sand filter) to achieve an enhanced removal rate for total suspended solids (TSS) removal under N.J.A.C. 7:8-5.5.
- 4. Additional design criteria for MTDs can be found in Chapter 11.3 of the New Jersey Stormwater Best Management Practices (NJ Stormwater BMP) Manual, which can be found online at <u>www.njstormwater.org</u>.
- 5. The maintenance plan for a site using this device shall incorporate, at a minimum, the maintenance requirements for the BarracudaTM MAX. A copy of the maintenance plan is attached to this certification. However, it is recommended to review the maintenance website at <u>https://assets.ads-pipe.com/m/2c834056a5a22888/original/Barracuda-Maintenance-Guide-MG1-01.pdf</u> for any changes to the maintenance requirements.
- 6. Sizing Requirement:

The example on the following page demonstrates the sizing procedure for the BarracudaTM MAX:

Example: A 0.25-acre impervious site is to be treated to 50% TSS removal using an BarracudaTM MAX treatment unit. The impervious site runoff (Q) based on the New Jersey Water Quality Design Storm was determined to be 0.79 cfs.

Maximum Treatment Flow Rate (MTFR) Evaluation:

The site runoff (Q) was based on the following: time of concentration = 10 minutes i = 3.2 in/hr (page 74, Fig. 5-16 of the NJ Stormwater BMP Manual) c = 0.99 (runoff coefficient for impervious) $Q = ciA = 0.99 \times 3.2 \times 0.25 = 0.79$ cfs

Given the site runoff is 0.79 cfs and based on Table A-1 below, the BarracudaTM MAX Model S3 with an MTFR of 0.85 cfs would be the smallest model that could be used for this site to remove 50% of the TSS from the impervious area without exceeding the MTFR.

The sizing table corresponding to the available system models is noted below. Additional specifications regarding each model can be found in the NJCAT Technology Verification Appendix under Tables A-1 and A-2.

Model	Manhole Diameter (ft)	Maximum Treatment Flow Rate (cfs)	50% Maximum Sediment Storage Area Volume (ft ³)
Barracuda MAX			
S3	3	0.85	5.89
Barracuda MAX			
S4	4	1.52	10.47
Barracuda MAX			
S5	5	2.37	16.36
Barracuda MAX			
S6	6	3.40	23.56
Barracuda MAX			
S8	8	6.08	41.89
Barracuda MAX			
S10	10	9.48	65.45

Table A-1 BarracudaTM MAX HDS Models and Associated MTFRs

A detailed maintenance plan is mandatory for any project with a stormwater BMP subject to the Stormwater Management rules under N.J.A.C. 7:8. The plan must include all of the items identified in the Maintenance requirements section of the Stormwater Management rules under N.J.A.C. 7:8-5.8. Such items include, but are not limited to, the list of inspection and maintenance equipment and tools, specific corrective and preventative maintenance tasks, indication of problems in the system, and training of maintenance personnel. Additional information can be found in Chapter 8: Maintenance and Retrofit of Stormwater Management Measures.

If you have any questions regarding the above information, please contact Lisa Schaefer of my office at <u>lisa.schaefer@dep.nj.gov</u>.

Sincerely,

abiel Mahon

Gabriel Mahon, Chief Bureau of NJPDES Stormwater Permitting & Water Quality Management

Attachment: Maintenance Plan

cc: Chron File Richard Magee, NJCAT Changi Wu, NJDEP-BFHSE Madhu Guru, NJDEP - BFHSE



USDA Natural Resources Conservation Service Web Soil Survey National Cooperative Soil Survey

MAP L	EGEND	MAP INFORMATION
Area of Interest (AOI)	🚍 Spoil Area	The soil surveys that comprise your AOI were mapped at
Area of Interest (AOI)	Stony Spot	1:24,000.
Soils	Very Stony Spot	Warning: Soil Map may not be valid at this scale.
Soil Map Unit Polygons	🅎 Wet Spot	Enlargement of maps beyond the scale of mapping can cause
Soil Map Unit Lines	other 3	misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of
Soil Map Unit Points	Special Line Features	contrasting soils that could have been shown at a more detailed
Special Point Features	Water Features	scale.
Blowout	Streams and Canals	Please rely on the bar scale on each map sheet for map
Borrow Pit	Transportation	measurements.
💥 Clay Spot	+++ Rails	Source of Map: Natural Resources Conservation Service
Closed Depression	nterstate Highways	Web Soil Survey URL: Coordinate System: Web Mercator (EPSG:3857)
Gravel Pit	JS Routes	Maps from the Web Soil Survey are based on the Web Mercato
Gravelly Spot	Major Roads	projection, which preserves direction and shape but distorts
🔕 Landfill	Local Roads	distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more
▲ Lava Flow	Background	accurate calculations of distance or area are required.
Marsh or swamp	Aerial Photography	This product is generated from the USDA-NRCS certified data a of the version date(s) listed below.
Mine or Quarry		Soil Survey Area: Rockland County, New York
Miscellaneous Water		Survey Area Data: Version 21, Sep 6, 2023
Perennial Water		Soil map units are labeled (as space allows) for map scales
V Rock Outcrop		1:50,000 or larger.
Saline Spot		Date(s) aerial images were photographed: May 31, 2022—Oc 27, 2022
Sandy Spot		The orthophoto or other base map on which the soil lines were
Severely Eroded Spot		compiled and digitized probably differs from the background
Sinkhole		imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.
Slide or Slip		
Sodic Spot		



Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
CrB	Cheshire gravelly fine sandy loam, 2 to 8 percent slopes	4.6	100.0%
Totals for Area of Interest		4.6	100.0%



Rockland County, New York

CrB—Cheshire gravelly fine sandy loam, 2 to 8 percent slopes

Map Unit Setting

National map unit symbol: 9v46 Elevation: 50 to 670 feet Mean annual precipitation: 47 to 50 inches Mean annual air temperature: 48 to 52 degrees F Frost-free period: 135 to 215 days Farmland classification: All areas are prime farmland

Map Unit Composition

Cheshire and similar soils: 80 percent Minor components: 20 percent Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Cheshire

Setting

Landform: Hills, till plains Landform position (two-dimensional): Summit Landform position (three-dimensional): Crest Down-slope shape: Convex Across-slope shape: Convex Parent material: Loamy till derived mostly from reddish sandstone, shale, and conglomerate

Typical profile

H1 - 0 to 10 inches: gravelly fine sandy loam *H2 - 10 to 22 inches:* gravelly fine sandy loam *H3 - 22 to 60 inches:* gravelly sandy loam

Properties and qualities

Slope: 3 to 8 percent Depth to restrictive feature: More than 80 inches Drainage class: Well drained Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high (0.57 to 5.95 in/hr) Depth to water table: More than 80 inches Frequency of flooding: None Frequency of ponding: None Available water supply, 0 to 60 inches: Moderate (about 8.1 inches)

Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 2e Hydrologic Soil Group: B Ecological site: F145XY013CT - Well Drained Till Uplands

USDA

Hydric soil rating: No

Minor Components

Watchaug

Percent of map unit: 5 percent Hydric soil rating: No

Cheshire, very stony Percent of map unit: 5 percent Hydric soil rating: No

Wethersfield

Percent of map unit: 5 percent *Hydric soil rating:* No

Yalesville

Percent of map unit: 5 percent *Hydric soil rating:* No

Data Source Information

Soil Survey Area: Rockland County, New York Survey Area Data: Version 21, Sep 6, 2023

WeinbergLim Engineering

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Appendix E

Barracuda® Max™ & Barracuda Maintenance Guide

One of Barracuda's advantages is the ease of maintenance. Like any system that collects pollutants, the Barracuda must be maintained for continued effectiveness. Maintenance is a simple procedure performed using a vacuum truck or similar equipment. The systems were designed to minimize the volume of water removed during routine maintenance, reducing disposal costs.

Contractors can access the pollutants stored in the manhole through the manhole cover. This allows them to gain vacuum hose access to the bottom of the manhole to remove sediment and trash. There is no confined space entry necessary for inspection or maintenance.

The entire maintenance procedure typically takes 2 to 4 hours, depending on the system's size, the captured material, and the vacuum truck's capacity.

Local regulations may apply to the maintenance procedure. Safe and legal disposal of pollutants is the responsibility of the maintenance contractor. Maintenance should be performed only by a qualified contractor.

Inspection and Cleaning Cycle

Periodic inspection is needed to determine the need for and frequency of maintenance. You should begin inspecting as soon as construction is complete and then on an annual basis. Typically, the system needs to be cleaned every 1-3 years.

Excessive oils, fuels or sediments may reduce the maintenance cycle. Periodic inspection is important.

Determining When to Clean

To determine the sediment depth, the maintenance contractor should lower a stadia rod into the manhole until it contacts the top of the captured sediment and mark that spot on the rod. Then push the probe through to the bottom of the sump and mark that spot to determine sediment depth.

Maintenance should occur when the sediment has reached the levels indicated in the Storage Capacity Chart.



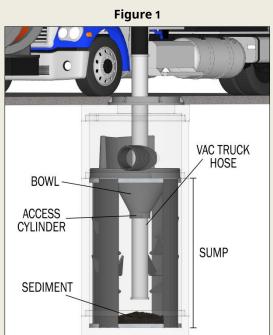
adspipe.com

Barracuda Storage Capacities

Model	Manhole Diameter in. (mm)	Total System Volume Gallons (Liters)	Treatment Chamber Capacity Gallons (Liters)	Standard Sediment Capacity (20" depth) Yards ³ (meters ³)	NJDEP Sediment Capacity (50% of standard depth) Yards ³ (meters ³)
S3	36 (900)	264 (999)	212 (803)	0.44 (0.34)	0.22 (0.17)
S4	48 (1200)	665 (2517)	564 (2135)	0.78 (0.60)	0.39 (0.30)
S5	60 (1500)	1040 (3937)	881 (3335)	1.21 (0.93)	0.61 (0.47)
S6	72 (1800)	1497 (5667)	1269 (4804)	1.75 (1.34)	0.88 (0.67)
S8	96 (2400)	4196 (15884)	3835 (14517)	3.10 (2.37)	1.55 (1.19)
S10	120 (3000)	7976 (30192)	7496 (28375)	4.85 (3.71)	2.43 (1.86)

Maintenance Instructions

- Remove the manhole cover to provide access to the pollutant storage. Pollutants are stored in the sump, below the bowl assembly visible from the surface. Access this area through the 8" (200 mm), 10" (250 mm), 15" (375 mm) or 20" (500 mm) diameter access cylinder.
- 2. Use a vacuum truck or other similar equipment to remove all water, debris, oils and sediment. See figure 1.
- 3. Use a high pressure hose to clean the manhole of all the remaining sediment and debris. Then, use the vacuum truck to remove the water.
- 4. Fill the cleaned manhole with water until the level reaches the invert of the outlet pipe.
- 5. Replace the manhole cover.
- 6. Dispose of the polluted water, oils, sediment and trash at an approved facility.
 - a. Local regulations prohibit the discharge of solid material into the sanitary system. Check with the local sewer authority for authority to discharge the liquid.
 - b. Some localities treat the pollutants as leachate. Check with local regulators about disposal requirements.
 - c. Additional local regulations may apply to the maintenance procedure.





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Maintenance Guide

BaySaver Barracuda[™]

One of the advantages of the BaySaver Barracuda is the ease of maintenance. Like any system that collects pollutants, the BaySaver Barracuda must be maintained for continued effectiveness. Maintenance is a simple procedure performed using a vacuum truck or similar equipment. The systems were designed to minimize the volume of water removed during routine maintenance, reducing disposal costs.

Contractors can access the pollutants stored in the manhole through the manhole cover. This allows them to gain vacuum hose access to the bottom of the manhole to remove sediment and trash. There is no confined space entry necessary for inspection or maintenance.

The entire maintenance procedure typically takes from 2 to 4 hours, depending on the size of the system, the captured material, and the capacity of the vacuum truck.

Local regulations may apply to the maintenance procedure. Safe and legal disposal of pollutants is the responsibility of the maintenance contractor. Maintenance should be performed only by a qualified contractor.

Inspection and Cleaning Cycle

Periodic inspection is needed to determine the need for and frequency of maintenance. You should begin inspecting as soon as construction is complete and thereafter on an annual basis. Typically, the system needs to be cleaned every 1-3 years.

Excessive oils, fuels or sediments may reduce the maintenance cycle. Periodic inspection is important.

Determining When to Clean

To determine the sediment depth, the maintenance contractor should lower a stadia rod into the manhole until it contacts the top of the captured sediment and mark that spot on the rod. Then push the probe through to the bottom of the sump and mark that spot to determine sediment depth.

Maintenance should occur when the sediment has reached the levels indicated in the Storage Capacity Chart.

BaySaver Barracuda Storage Capacities

Model	Manhole Diameter	Treatment Chamber Capacity	Standard Sediment Capacity (20" depth)	NJDEP Sediment Capacity (50% of standard depth)
S3	36"	212 gallons	0.44 cubic yards	0.22 cubic yards
S4	48"	564 gallons	0.78 cubic yards	0.39 cubic yards
S5	60"	881 gallons	1.21 cubic yards	0.61 cubic yards
S6	72"	1269 gallons	1.75 cubic yards	0.88 cubic yards
S8	96"	3835 gallons	3.10 cubic yards	1.55 cubic yards
S10	120"	7496 gallons	4.85 cubic yards	2.43 cubic yards

Maintenance Instructions

1. Remove the manhole cover to provide access to the pollutant storage. Pollutants are stored in the sump, below the bowl assembly visible from the surface. You'll access this area through the 10" diameter access cylinder.



- 2. Use a vacuum truck or other similar equipment to remove all water, debris, oils and sediment. See figure 1.
- 3. Use a high pressure hose to clean the manhole of all the remaining sediment and debris. Then, use the vacuum truck to remove the water.
- 4. Fill the cleaned manhole with water until the level reaches the invert of the outlet pipe.
- 5. Replace the manhole cover.
- 6. Dispose of the polluted water, oils, sediment and trash at an approved facility.
 - Local regulations prohibit the discharge of solid material into the sanitary system. Check with the local sewer authority for authority to discharge the liquid.
 - Some localities treat the pollutants as leachate. Check with local regulators about disposal requirements.
 - Additional local regulations may apply to the maintenance procedure.

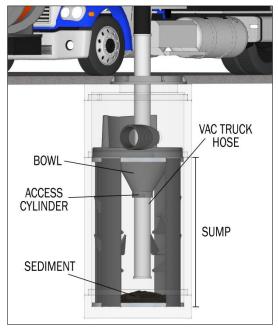


Figure 1